



REGIONE PUGLIA COMUNE DI ACCADIA



COMPLETAMENTO DISSESTO IDROGEOLOGICO CENTRO URBANO VIA PERTINI

FINANZIAMENTO

P.O.R. Puglia 2014/2020 - Asse V - Azione 5.1 - "Interventi di riduzione del rischio idrogeologico e di erosione costiera". Programma di interventi cantierabili ai sensi della DGR n. 511 del 19.04.2016

Progetto N. 17328
Ottobre 2017

Progetto _____ **BONIFICA DISSESTO**

Fase Progettuale _____ PROGETTO ESECUTIVO

Localizzazione _____ ACCADIA - Via Pertini

Committente _____ COMUNE DI ACCADIA

Responsabile Unico del Procedimento _____ DOTT. ING. GIUSEPPE CELA

Progettista Raggruppamento temporaneo di professionisti

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rev.	data	aggiornamento	redatto	verificato	approvato
00	Ottobre 2017	Emissione	Bruschini	Bartocci	Balducci

Verifica terre rinforzate
Fascicolo dei calcoli

scala

tav.


GEOTB

	VERIFICA TERRE RINFORZATE - FASCICOLO DEI CALCOLI		
	Nome File: 17328GEOTB PE00	N° Pratica: 17328	Pagina 1

COMUNE DI ACCADIA (FG)

PROGETTO INTERVENTO DI COMPLETAMENTO DISSESTO IDROGEOLOGICO CENTRO URBANO VIA PERTINI

VERIFICA TERRE RINFORZATE - FASCICOLO DEI CALCOLI

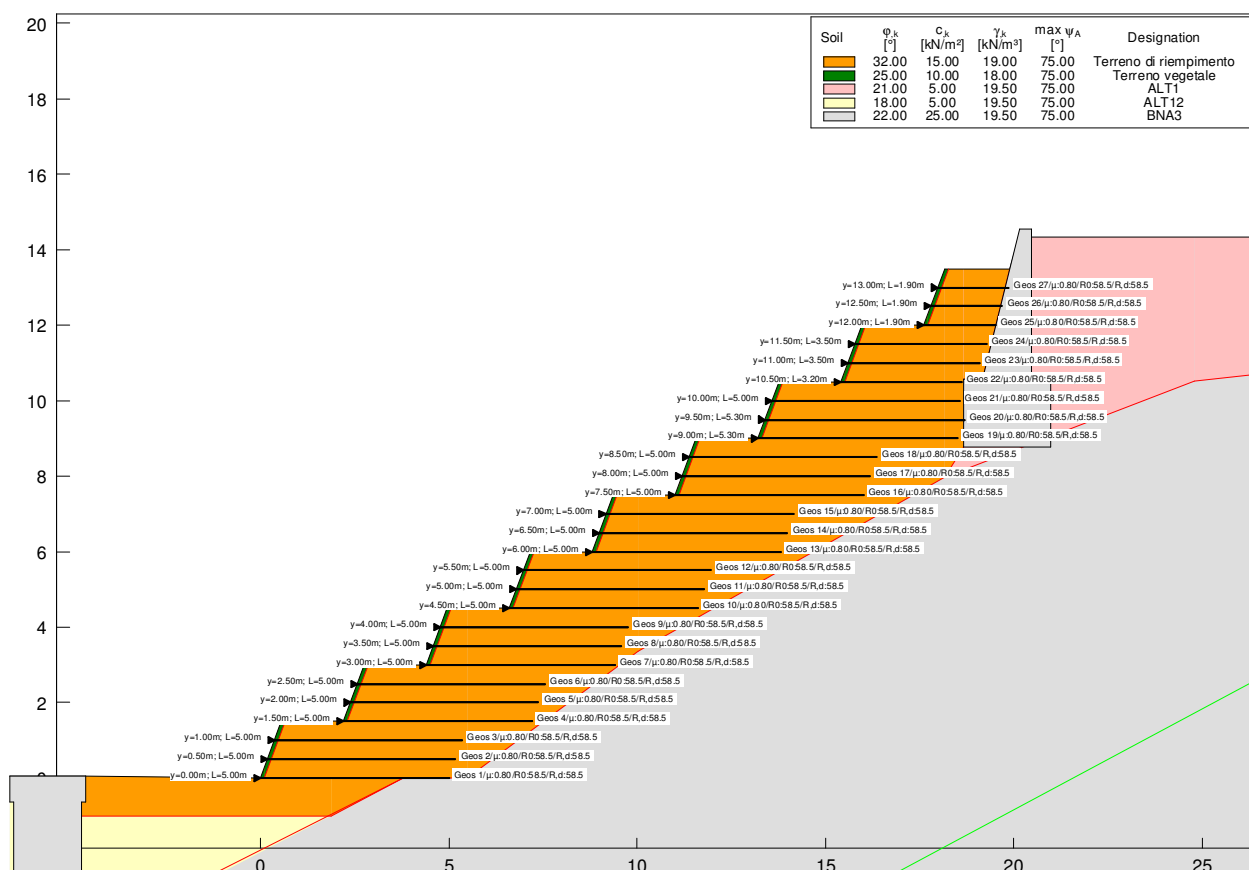
	VERIFICA TERRE RINFORZATE - FASCICOLO DEI CALCOLI		
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SEZIONE 2
**PROGETTO: ACCADIA
CONFIGURAZIONE STRUTTURALE
SEZIONE 2**

Geosintetici di rinforzo				
Strato n°	Tipo	Tamm	Lungh.	Spaz.
[-]	[-]	[kN/m]	[m]	[m]
1-6	Fortrac 110/25-20/30 MPT	58,53	5.50	0.50
7-12	Fortrac 110/25-20/30 MPT	58,53	5.00	0.50
13-18	Fortrac 110/25-20/30 MPT	58,53	5.00	0.50
19-20	Fortrac 110/25-20/30 MPT	58,53	5.30	0.50
21	Fortrac 110/25-20/30 MPT	58,53	5.00	0.50
22	Fortrac 110/25-20/30 MPT	58,53	3.20	0.50
23-24	Fortrac 110/25-20/30 MPT	58,53	3.50	0.50
25-27	Fortrac 110/25-20/30 MPT	58,53	1.90	0.50



**PROGETTO: ACCADIA
VERIFICA DI STABILITÀ INTERNA E COMPOSTA
SEZIONE 2**

NTC2008

Combinazione:

A2+M2+R2

$$E_d \leq (R / \gamma_R)$$

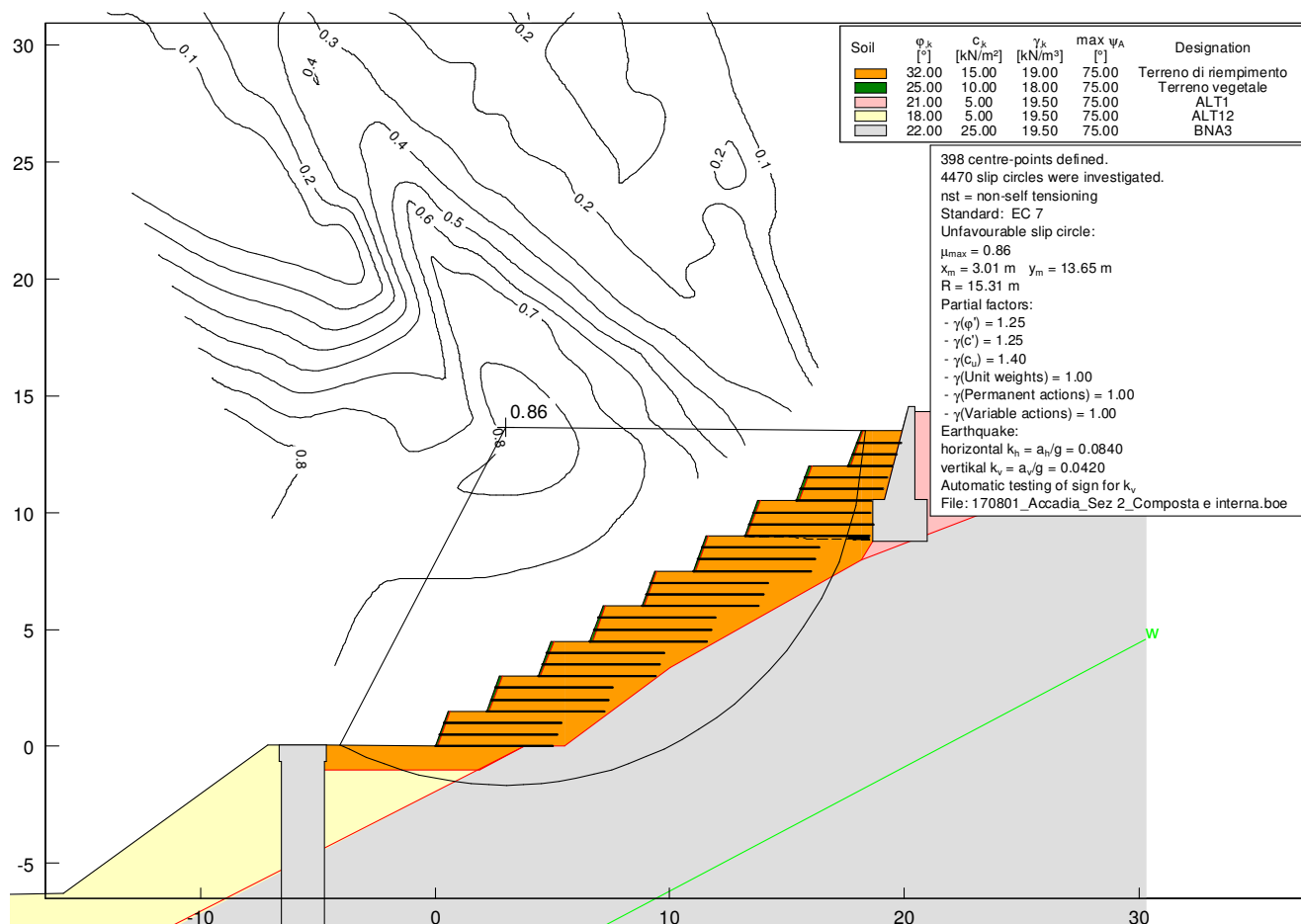
$\gamma_R = 1,1$ (tab. 6.8.I NTC2008)

$\rightarrow R/E_d \geq 1,1 \rightarrow F_s \geq 1,1$.

μ : grado di utilizzo del sistema

$\mu = 1 / F_s = 0,86$

$\rightarrow F_s = 1,16 \geq 1,10$



Slope stability analysis to EC 7
using circular slip surfaces

Datei: 170801_Accadia_Sez 2_Composta e interna.boe

Parameter list

phi [°] = friction angle

c [kN/m²] = cohesion

gamma [kN/m³] = unit weight

Max. psi(A) [°] = angle between slip direction of failure mechanism and the tension member

μ [-] = utilisation factor

xm,ym [m] = x and y values of slip circle centre-point

rad [m] = slip circle radius

Partial factors: (GEO-3)

- gam(phi)= 1.25

- gam(c') = 1.25

- gam(cu) = 1.40

- gam(Unit weights) = 1.00

- gam(Permanent actions) = 1.00

- gam(Variable actions) = 1.00

Movement direction of slip body to the Left

Coordinates of surface points

No.	x	y
[-]	[m]	[m]
1	-40.000	-16.247
2	-22.291	-9.046
3	-18.422	-6.377
4	-15.871	-6.270
5	-7.148	0.060
6	-4.641	0.060
7	0.000	0.000
8	0.546	1.500
9	2.200	1.500
10	2.746	3.000
11	4.400	3.000
12	4.946	4.500
13	6.600	4.500
14	7.146	6.000
15	8.800	6.000
16	9.346	7.500
17	11.000	7.500
18	11.546	9.000
19	13.200	9.000
20	13.746	10.500
21	15.400	10.500
22	15.946	12.000
23	17.600	12.000
24	18.146	13.500
25	19.890	13.500
26	20.115	14.339
27	30.334	14.339

Characteristic soil properties

Soil	phi,k	c,k	gamma,k	max psi(A)	drained	Designation
[-]	[°]	[kN/m²]	[kN/m³]	[°]		
1	32.00	15.00	19.00	75.00	ja	Terreno di riempimento
2	25.00	10.00	18.00	75.00	ja	Terreno vegetale
3	21.00	5.00	19.50	75.00	ja	ALT1
4	18.00	5.00	19.50	75.00	ja	ALT12
5	22.00	25.00	19.50	75.00	ja	BN3

Design soil properties

Soil	phi,d	c,d	gamma,d	drained	Designation
[-]	[°]	[kN/m²]	[kN/m³]		
1	26.56	12.00	19.00	ja	Terreno di riempimento
2	20.46	8.00	18.00	ja	Terreno vegetale
3	17.07	4.00	19.50	ja	ALT1

4	14.57	4.00	19.50	ja	ALT12
5	17.91	20.00	19.50	ja	BNA3

Coordinates of layers and soil numbers

No.	x(left)	y(left)	x(right)	y(right)	Soil no.
[-]	[m]	[m]	[m]	[m]	
1	0.000	0.000	0.100	0.000	2
2	0.100	0.000	0.652	1.500	2
3	2.200	1.500	2.300	1.500	2
4	2.300	1.500	2.852	3.000	2
5	4.400	3.000	4.500	3.000	2
6	4.500	3.000	5.052	4.500	2
7	6.600	4.500	6.700	4.500	2
8	6.700	4.500	7.252	6.000	2
9	8.800	6.000	8.900	6.000	2
10	8.900	6.000	9.452	7.500	2
11	11.000	7.500	11.100	7.500	2
12	11.100	7.500	11.652	9.000	2
13	13.200	9.000	13.300	9.000	2
14	13.300	9.000	13.852	10.500	2
15	15.400	10.500	15.500	10.500	2
16	15.500	10.500	16.052	12.000	2
17	17.600	12.000	17.700	12.000	2
18	17.700	12.000	18.252	13.500	2
19	0.100	0.000	5.500	0.000	1
20	5.500	0.000	10.031	3.359	1
21	10.031	3.359	18.159	7.988	1
22	18.159	7.988	18.655	8.762	1
23	18.655	10.562	19.155	10.562	1
24	19.155	10.562	19.890	13.500	1
25	18.159	7.988	24.790	10.524	3
26	24.790	10.524	30.370	11.060	3
27	-40.000	-21.251	-26.335	-15.662	4
28	-4.741	-1.000	1.877	-1.000	1
29	1.877	-1.000	3.801	0.000	1
30	-26.335	-15.662	3.801	0.000	4
31	-40.000	-40.000	30.334	-40.000	5

Coordinates of pwp polygon course

No.	x	y
[-]	[m]	[m]
1	-40.000	-25.672
2	-30.726	-22.102
3	-5.138	-14.376
4	30.280	4.595

Geosynthetics

Adhesive force f calculated with:

$$f = \mu \cdot \tan(\phi) \cdot \sigma'$$

 μ [-] = reduction factor for friction between ground and geosynthetics

 σ' [kN/m²] = effekive Spannung

R0 [kN/m] = design force at connection

L0 [m] = fold-back length

R,d [kN/m] = acceptable design force

Nr.	x1	y1	x2	y2	μ	R0	R, d
[-]	[m]	[m]	[m]	[m]	[-]	[kN/m]	[kN/m]
1	0.00	0.00	5.00	0.00	0.800	58.53	58.53
2	0.18	0.50	5.18	0.50	0.800	58.53	58.53
3	0.36	1.00	5.36	1.00	0.800	58.53	58.53
4	2.20	1.50	7.20	1.50	0.800	58.53	58.53
5	2.38	2.00	7.38	2.00	0.800	58.53	58.53
6	2.56	2.50	7.56	2.50	0.800	58.53	58.53
7	4.40	3.00	9.40	3.00	0.800	58.53	58.53
8	4.58	3.50	9.58	3.50	0.800	58.53	58.53
9	4.76	4.00	9.76	4.00	0.800	58.53	58.53
10	6.60	4.50	11.60	4.50	0.800	58.53	58.53
11	6.78	5.00	11.78	5.00	0.800	58.53	58.53
12	6.96	5.50	11.96	5.50	0.800	58.53	58.53
13	8.80	6.00	13.80	6.00	0.800	58.53	58.53

14	8.98	6.50	13.98	6.50	0.800	58.53	58.53
15	9.16	7.00	14.16	7.00	0.800	58.53	58.53
16	11.00	7.50	16.00	7.50	0.800	58.53	58.53
17	11.18	8.00	16.18	8.00	0.800	58.53	58.53
18	11.36	8.50	16.36	8.50	0.800	58.53	58.53
19	13.20	9.00	18.50	9.00	0.800	58.53	58.53
20	13.38	9.50	18.68	9.50	0.800	58.53	58.53
21	13.56	10.00	18.56	10.00	0.800	58.53	58.53
22	15.40	10.50	18.60	10.50	0.800	58.53	58.53
23	15.58	11.00	19.08	11.00	0.800	58.53	58.53
24	15.76	11.50	19.26	11.50	0.800	58.53	58.53
25	17.60	12.00	19.50	12.00	0.800	58.53	58.53
26	17.78	12.50	19.68	12.50	0.800	58.53	58.53
27	17.96	13.00	19.86	13.00	0.800	58.53	58.53

Structural element 1

No.	x	y
[-]	[m]	[m]
1	-6.64	0.06
2	-4.64	0.06
3	-4.64	-0.64
4	-4.74	-0.64
5	-4.74	-11.94
6	-6.54	-11.94
7	-6.54	-0.64
8	-6.64	-0.64

Structural element 2

No.	x	y
[-]	[m]	[m]
1	20.16	14.56
2	20.46	14.56
3	20.46	10.56
4	20.96	10.56
5	20.96	8.76
6	18.66	8.76
7	18.66	10.56
8	19.16	10.56

Earthquake

horizontal $k_h = a_h/g = 0.0840$

vertical $k_v = a_v/g = 0.0420$

Automatic testing of sign for k_v
 k_v (governing) = -0.0420

(a_h = horizontal seismic acceleration in m/s^2)

(a_v = vertical seismic acceleration in m/s^2)

(g = gravitational acceleration = $9.81 m/s^2$)

Water level in front of slope left [m] = -20.00

Water level in front of slope right [m] = -20.00

Gamma water [kN/m^3] = 10.000

Calculation with consideration of passive earth pressure wedge

Slip circle no. 308

 $\mu = 0.8568 = [M(G_i) + M(S)] / [M(T_i) + M(R)]$
 $x_m = 3.0089$
 $y_m = 13.6454$

Radius = 15.3123

Numerator = 15555.6701

Denominator = 18156.3182

 $M(T_i) = 18156.3182$
 $M(R) = 0.0000$
 $M(G_i) = 13882.5064$
 $M(S) = 1673.1638$

Slice values

x = x (slice toe)
y = y (slice toe)
b = slice width
phi = friction angle
c = cohesion
PWP = pore water press. coeff.
tet = inclination of slice
g = weight
n = normal force
t = tangential force
FAi/FA0i/Rsi = see Equations (7) and (8) in DIN 4084:2009
pw = pore water pressure
pw(con) = excess pwp due to consolidation
wv = vertical water pressure
pst = permanent loads and footing
pv = live loads
fakpv = factor for live loads
So No. = soil number

Nr.	x	y	b	phi,d	c,d	PWD	tet	g,k	n	t	FAi	FA0i	Rsi
pw	pw(kon)	wv	pst,d	pv,d	fakpv	Bo-Nr.							
[kN/m]	[kN/m]	[m]	[m]	[m]	[°]	[kN/m²]	[°]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]
0.0	0.0	-3.668	-0.135	0.745	26.6	12.0	0.00	25.9	2.6	3.8	14.4	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	28							
0.0	0.0	-2.922	-0.472	0.745	26.6	12.0	0.00	22.8	7.2	9.9	16.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	28							
0.0	0.0	-2.177	-0.762	0.745	26.6	12.0	0.00	19.8	11.2	14.7	18.6	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	28							
0.0	0.0	-1.432	-1.009	0.745	14.6	4.0	0.00	16.9	14.6	17.0	7.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	30							
0.0	0.0	-0.686	-1.214	0.745	14.6	4.0	0.00	14.0	17.4	19.8	8.4	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	30							
0.0	0.0	0.059	-1.380	0.745	14.6	4.0	0.00	11.1	21.9	24.3	9.5	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	30							
0.0	0.0	0.804	-1.507	0.745	14.6	4.0	0.00	8.3	42.8	46.6	15.2	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	30							
0.0	0.0	1.550	-1.597	0.745	17.9	20.0	0.00	5.5	44.1	47.4	30.7	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	2.295	-1.650	0.745	17.9	20.0	0.00	2.7	48.4	51.2	31.7	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	3.041	-1.667	0.745	17.9	20.0	0.00	-0.1	66.6	69.3	37.3	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	3.786	-1.647	0.745	17.9	20.0	0.00	-2.9	66.4	68.3	36.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	4.531	-1.591	0.745	17.9	20.0	0.00	-5.7	70.5	71.9	37.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	5.277	-1.498	0.745	17.9	20.0	0.00	-8.5	85.5	86.5	42.4	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	6.022	-1.368	0.745	17.9	20.0	0.00	-11.3	83.8	84.3	41.7	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	6.767	-1.199	0.745	17.9	20.0	0.00	-14.2	87.8	88.2	42.9	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	7.513	-0.990	0.745	17.9	20.0	0.00	-17.1	99.9	100.4	46.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	0	31							
0.0	0.0	8.258	-0.739	0.745	17.9	20.0	0.00	-20.0	96.5	97.2	45.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	9.003	-0.445	0.745	17.9	20.0	0.00	-23.0	100.1	101.4	47.3	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	9.749	-0.104	0.745	17.9	20.0	0.00	-26.1	108.9	111.3	50.6	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	10.494	0.287	0.745	17.9	20.0	0.00	-29.3	103.4	106.9	49.3	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	11.240	0.733	0.745	17.9	20.0	0.00	-32.5	106.2	111.5	51.1	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	11.985	1.240	0.745	17.9	20.0	0.00	-35.9	111.1	119.0	53.8	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	12.730	1.815	0.745	17.9	20.0	0.00	-39.4	102.9	113.1	52.3	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	13.476	2.469	0.745	17.9	20.0	0.00	-43.1	104.1	118.0	54.4	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	14.221	3.217	0.745	17.9	20.0	0.00	-47.1	104.1	122.7	56.5	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	14.966	4.081	0.745	17.9	20.0	0.00	-51.3	91.7	113.6	54.5	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	15.712	5.095	0.745	17.9	20.0	0.00	-56.1	89.0	117.7	57.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	16.457	6.323	0.745	17.9	20.0	0.00	-61.4	80.7	116.5	58.3	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31							
0.0	0.0	17.203	7.900	0.745	26.6	12.0	0.00	-68.0	58.1	78.3	50.7	55.2	0.0
0.0	0.0	0.0	0.0	0.0	1	21							
0.0	0.0	17.948	10.285	0.745	26.6	12.0	0.00	-77.3	37.6	61.5	44.8	0.0	0.0

0.0 0.0 0.0 0.0 0.0 1 21

0.0 0.0 0.0 0.0 0.0

2064.9

Geosynthetic 1
x1 = 0.000 m
y1 = 0.000 m
x2 = 5.000 m
y2 = 0.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 2
x1 = 0.182 m
y1 = 0.500 m
x2 = 5.182 m
y2 = 0.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 3
x1 = 0.364 m
y1 = 1.000 m
x2 = 5.364 m
y2 = 1.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 4
x1 = 2.200 m
y1 = 1.500 m
x2 = 7.200 m
y2 = 1.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 5
x1 = 2.382 m
y1 = 2.000 m
x2 = 7.382 m
y2 = 2.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 6
x1 = 2.564 m
y1 = 2.500 m
x2 = 7.564 m
y2 = 2.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 7
x1 = 4.400 m
y1 = 3.000 m
x2 = 9.400 m
y2 = 3.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 8

x1 = 4.582 m
y1 = 3.500 m
x2 = 9.582 m
y2 = 3.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 9
x1 = 4.764 m
y1 = 4.000 m
x2 = 9.764 m
y2 = 4.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 10
x1 = 6.600 m
y1 = 4.500 m
x2 = 11.600 m
y2 = 4.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 11
x1 = 6.782 m
y1 = 5.000 m
x2 = 11.782 m
y2 = 5.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 12
x1 = 6.964 m
y1 = 5.500 m
x2 = 11.964 m
y2 = 5.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 13
x1 = 8.800 m
y1 = 6.000 m
x2 = 13.800 m
y2 = 6.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 14
x1 = 8.982 m
y1 = 6.500 m
x2 = 13.982 m
y2 = 6.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 15
x1 = 9.164 m
y1 = 7.000 m
x2 = 14.164 m
y2 = 7.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 16
 $x1 = 11.000 \text{ m}$
 $y1 = 7.500 \text{ m}$
 $x2 = 16.000 \text{ m}$
 $y2 = 7.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 17
 $x1 = 11.182 \text{ m}$
 $y1 = 8.000 \text{ m}$
 $x2 = 16.182 \text{ m}$
 $y2 = 8.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 18
 $x1 = 11.364 \text{ m}$
 $y1 = 8.500 \text{ m}$
 $x2 = 16.364 \text{ m}$
 $y2 = 8.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 19
 $x1 = 13.200 \text{ m}$
 $y1 = 9.000 \text{ m}$
 $x2 = 18.500 \text{ m}$
 $y2 = 9.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 67.96^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 55.196 kN/m
Bonded length = 0.90 m
 $F(AL) = 196.29 \text{ kN/m}$
 $F(AR) = 55.20 \text{ kN/m}$

Geosynthetic 20
 $x1 = 13.382 \text{ m}$
 $y1 = 9.500 \text{ m}$
 $x2 = 18.682 \text{ m}$
 $y2 = 9.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 77.32^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 21
 $x1 = 13.564 \text{ m}$
 $y1 = 10.000 \text{ m}$
 $x2 = 18.564 \text{ m}$
 $y2 = 10.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 77.32^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009

Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 22

x1 = 15.400 m

y1 = 10.500 m

x2 = 18.600 m

y2 = 10.500 m

$\mu = 0.800$

R0 = 58.530 kN/m

R,d = 58.530 kN/m

Work. psi(A) = 77.32 °

Limit psi(A) = 75.00 °

Geosynthetic is not self-tensioning nach DIN 4084:2009

Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 23

x1 = 15.582 m

y1 = 11.000 m

x2 = 19.082 m

y2 = 11.000 m

$\mu = 0.800$

R0 = 58.530 kN/m

R,d = 58.530 kN/m

Work. psi(A) = 77.32 °

Limit psi(A) = 75.00 °

Geosynthetic is not self-tensioning nach DIN 4084:2009

Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 24

x1 = 15.764 m

y1 = 11.500 m

x2 = 19.264 m

y2 = 11.500 m

$\mu = 0.800$

R0 = 58.530 kN/m

R,d = 58.530 kN/m

Work. psi(A) = 77.32 °

Limit psi(A) = 75.00 °

Geosynthetic is not self-tensioning nach DIN 4084:2009

Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 25

x1 = 17.600 m

y1 = 12.000 m

x2 = 19.500 m

y2 = 12.000 m

$\mu = 0.800$

R0 = 58.530 kN/m

R,d = 58.530 kN/m

Work. psi(A) = 77.32 °

Limit psi(A) = 75.00 °

Geosynthetic is not self-tensioning nach DIN 4084:2009

Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 26

x1 = 17.782 m

y1 = 12.500 m

x2 = 19.682 m

y2 = 12.500 m

$\mu = 0.800$

R0 = 58.530 kN/m

R,d = 58.530 kN/m

Work. psi(A) = 77.32 °

Limit psi(A) = 75.00 °

Geosynthetic is not self-tensioning nach DIN 4084:2009

Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 27

x1 = 17.964 m

$y1 = 13.000 \text{ m}$
 $x2 = 19.864 \text{ m}$
 $y2 = 13.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$
 $\text{Work. psi(A)} = 77.32^\circ$
 $\text{Limit psi(A)} = 75.00^\circ$
 Geosynthetic is not self-tensioning nach DIN 4084:2009
 Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

PROGETTO: ACCADIA
VERIFICA DI STABILITÀ SEZIONE 2
Verifica di stabilità globale

NTC2008

Combinazione:

A2+M2+R2

$$E_d \leq (R / \gamma_R)$$

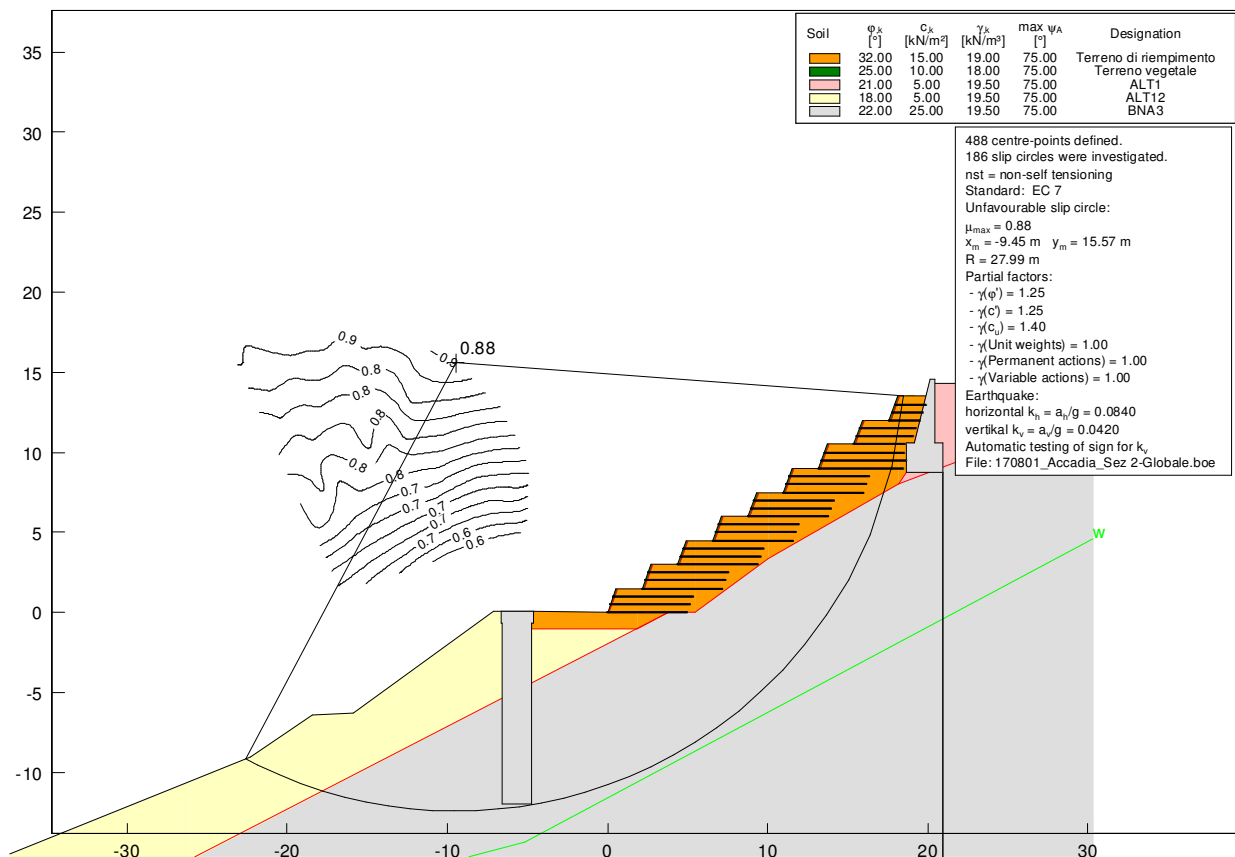
$\gamma_R = 1,1$ (tab. 6.8.I NTC2008)

$\rightarrow R/E_d \geq 1,1 \rightarrow F_s \geq 1,1$

μ : grado di utilizzo del sistema

$\mu = 1 / F_s = 0,88$

$\rightarrow F_s = 1,14 \geq 1,10$



Slope stability analysis to EC 7
using circular slip surfaces

Datei: 170801_Accadia_Sez 2-Globale.boe

Parameter list

phi [°] = friction angle

c [kN/m²] = cohesion

gamma [kN/m³] = unit weight

Max. psi(A) [°] = angle between slip direction of failure mechanism and the tension member

μ [-] = utilisation factor

xm,ym [m] = x and y values of slip circle centre-point

rad [m] = slip circle radius

Partial factors: (GEO-3)

- gam(phi)= 1.25

- gam(c') = 1.25

- gam(cu) = 1.40

- gam(Unit weights) = 1.00

- gam(Permanent actions) = 1.00

- gam(Variable actions) = 1.00

Movement direction of slip body to the Left

Coordinates of surface points

No.	x	y
[-]	[m]	[m]
1	-40.000	-16.247
2	-22.291	-9.046
3	-18.422	-6.377
4	-15.871	-6.270
5	-7.148	0.060
6	-4.641	0.060
7	0.000	0.000
8	0.546	1.500
9	2.200	1.500
10	2.746	3.000
11	4.400	3.000
12	4.946	4.500
13	6.600	4.500
14	7.146	6.000
15	8.800	6.000
16	9.346	7.500
17	11.000	7.500
18	11.546	9.000
19	13.200	9.000
20	13.746	10.500
21	15.400	10.500
22	15.946	12.000
23	17.600	12.000
24	18.146	13.500
25	19.890	13.500
26	20.115	14.339
27	30.334	14.339

Characteristic soil properties

Soil	phi,k	c,k	gamma,k	max psi(A)	drained	Designation
[-]	[°]	[kN/m²]	[kN/m³]	[°]		
1	32.00	15.00	19.00	75.00	ja	Terreno di riempimento
2	25.00	10.00	18.00	75.00	ja	Terreno vegetale
3	21.00	5.00	19.50	75.00	ja	ALT1
4	18.00	5.00	19.50	75.00	ja	ALT12
5	22.00	25.00	19.50	75.00	ja	BNA3

Design soil properties

Soil	phi,d	c,d	gamma,d	drained	Designation
[-]	[°]	[kN/m²]	[kN/m³]		
1	26.56	12.00	19.00	ja	Terreno di riempimento
2	20.46	8.00	18.00	ja	Terreno vegetale
3	17.07	4.00	19.50	ja	ALT1

4	14.57	4.00	19.50	ja	ALT12
5	17.91	20.00	19.50	ja	BNA3

Coordinates of layers and soil numbers

No.	x(left)	y(left)	x(right)	y(right)	Soil no.
[-]	[m]	[m]	[m]	[m]	
1	0.000	0.000	0.100	0.000	2
2	0.100	0.000	0.652	1.500	2
3	2.200	1.500	2.300	1.500	2
4	2.300	1.500	2.852	3.000	2
5	4.400	3.000	4.500	3.000	2
6	4.500	3.000	5.052	4.500	2
7	6.600	4.500	6.700	4.500	2
8	6.700	4.500	7.252	6.000	2
9	8.800	6.000	8.900	6.000	2
10	8.900	6.000	9.452	7.500	2
11	11.000	7.500	11.100	7.500	2
12	11.100	7.500	11.652	9.000	2
13	13.200	9.000	13.300	9.000	2
14	13.300	9.000	13.852	10.500	2
15	15.400	10.500	15.500	10.500	2
16	15.500	10.500	16.052	12.000	2
17	17.600	12.000	17.700	12.000	2
18	17.700	12.000	18.252	13.500	2
19	0.100	0.000	5.500	0.000	1
20	5.500	0.000	10.031	3.359	1
21	10.031	3.359	18.159	7.988	1
22	18.159	7.988	18.655	8.762	1
23	18.655	10.562	19.155	10.562	1
24	19.155	10.562	19.890	13.500	1
25	18.159	7.988	24.790	10.524	3
26	24.790	10.524	30.370	11.060	3
27	-40.000	-21.251	-26.335	-15.662	4
28	-4.741	-1.000	1.877	-1.000	1
29	1.877	-1.000	3.801	0.000	1
30	-26.335	-15.662	3.801	0.000	4
31	-40.000	-40.000	30.334	-40.000	5

Coordinates of pwp polygon course

No.	x	y
[-]	[m]	[m]
1	-40.000	-25.672
2	-30.726	-22.102
3	-5.138	-14.376
4	30.280	4.595

Geosynthetics

Adhesive force f calculated with:

$$f = \mu \cdot \tan(\phi) \cdot \sigma'$$

 μ [-] = reduction factor for friction between ground and geosynthetics

 σ' [kN/m²] = effekive Spannung

R0 [kN/m] = design force at connection

L0 [m] = fold-back length

R,d [kN/m] = acceptable design force

Nr.	x1	y1	x2	y2	μ	R0	R, d
[-]	[m]	[m]	[m]	[m]	[-]	[kN/m]	[kN/m]
1	0.00	0.00	5.00	0.00	0.800	58.53	58.53
2	0.18	0.50	5.18	0.50	0.800	58.53	58.53
3	0.36	1.00	5.36	1.00	0.800	58.53	58.53
4	2.20	1.50	7.20	1.50	0.800	58.53	58.53
5	2.38	2.00	7.38	2.00	0.800	58.53	58.53
6	2.56	2.50	7.56	2.50	0.800	58.53	58.53
7	4.40	3.00	9.40	3.00	0.800	58.53	58.53
8	4.58	3.50	9.58	3.50	0.800	58.53	58.53
9	4.76	4.00	9.76	4.00	0.800	58.53	58.53
10	6.60	4.50	11.60	4.50	0.800	58.53	58.53
11	6.78	5.00	11.78	5.00	0.800	58.53	58.53
12	6.96	5.50	11.96	5.50	0.800	58.53	58.53
13	8.80	6.00	13.80	6.00	0.800	58.53	58.53

14	8.98	6.50	13.98	6.50	0.800	58.53	58.53
15	9.16	7.00	14.16	7.00	0.800	58.53	58.53
16	11.00	7.50	16.00	7.50	0.800	58.53	58.53
17	11.18	8.00	16.18	8.00	0.800	58.53	58.53
18	11.36	8.50	16.36	8.50	0.800	58.53	58.53
19	13.20	9.00	18.50	9.00	0.800	58.53	58.53
20	13.38	9.50	18.68	9.50	0.800	58.53	58.53
21	13.56	10.00	18.56	10.00	0.800	58.53	58.53
22	15.40	10.50	18.60	10.50	0.800	58.53	58.53
23	15.58	11.00	19.08	11.00	0.800	58.53	58.53
24	15.76	11.50	19.26	11.50	0.800	58.53	58.53
25	17.60	12.00	19.50	12.00	0.800	58.53	58.53
26	17.78	12.50	19.68	12.50	0.800	58.53	58.53
27	17.96	13.00	19.86	13.00	0.800	58.53	58.53

Structural element 1

No.	x	y
[-]	[m]	[m]
1	-6.64	0.06
2	-4.64	0.06
3	-4.64	-0.64
4	-4.74	-0.64
5	-4.74	-11.94
6	-6.54	-11.94
7	-6.54	-0.64
8	-6.64	-0.64

Structural element 2

No.	x	y
[-]	[m]	[m]
1	20.16	14.56
2	20.46	14.56
3	20.46	10.56
4	20.96	10.56
5	20.96	8.76
6	18.66	8.76
7	18.66	10.56
8	19.16	10.56

Structural element 3

No.	x	y
[-]	[m]	[m]
1	20.96	8.76
2	20.96	-40.00

Earthquake

horizontal $k_h = a_h/g = 0.0840$

vertical $k_v = a_v/g = 0.0420$

Automatic testing of sign for k_v
 k_v (governing) = -0.0420

(a_h = horizontal seismic acceleration in m/s^2)

(a_v = vertical seismic acceleration in m/s^2)

(g = gravitational acceleration = $9.81 m/s^2$)

Water level in front of slope left [m] = -20.00

Water level in front of slope right [m] = -20.00

Gamma water $[kN/m^3] = 10.000$

Calculation with consideration of passive earth pressure wedge

Slip circle no. 221

 $\mu = 0.8823 = [M(G_i) + M(S)] / [M(T_i) + M(R)]$
 $x_m = -9.4499$
 $y_m = 15.5744$

Radius = 27.9908

Numerator = 84525.1582

Denominator = 95800.9981

M(Ti) = 95800.9981
M(R) = 0.0000
M(Gi) = 72932.6447
M(S) = 11592.5135

Slice values

x = x (slice toe)
y = y (slice toe)
b = slice width
phi = friction angle
c = cohesion
PWP = pore water press. coeff.
tet = inclination of slice
g = weight
n = normal force
t = tangential force
FAi/FA0i/Rsi = see Equations (7) and (8) in DIN 4084:2009
pw = pore water pressure
pw(con) = excess pwp due to consolidation
wv = vertical water pressure
pst = permanent loads and footing
pv = live loads
fakpv = factor for live loads
So No. = soil number

	Nr.	x	y	b	phi,d	c,d	PWD	tet	g,k	n	t	FAi	FA0i	Rsi
pw	pw(kon)	wv	pst,d	pv,d	fakpv	Bo-Nr.								
[kN/m]	[kN/m]	[kN/m]	[m]	[m]	[°]	[kN/m²]	[-]	[°]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]
0.0	0.0	1	-21.877	-9.506	1.367	14.6	4.0	0.00	26.4	19.9	26.1	13.7	0.0	0.0
0.0	0.0	2	-20.510	-10.139	1.367	14.6	4.0	0.00	23.3	61.9	77.9	26.9	0.0	0.0
0.0	0.0	3	-19.142	-10.685	1.367	14.6	4.0	0.00	20.3	101.6	123.3	38.4	0.0	0.0
0.0	0.0	4	-17.775	-11.150	1.367	14.6	4.0	0.00	17.3	128.0	150.4	45.3	0.0	0.0
0.0	0.0	5	-16.407	-11.538	1.367	17.9	20.0	0.00	14.4	139.9	162.4	82.9	0.0	0.0
0.0	0.0	6	-15.040	-11.853	1.367	17.9	20.0	0.00	11.5	165.0	186.2	89.8	0.0	0.0
0.0	0.0	7	-13.672	-12.096	1.367	17.9	20.0	0.00	8.7	197.9	218.1	99.4	0.0	0.0
0.0	0.0	8	-12.305	-12.270	1.367	17.9	20.0	0.00	5.9	229.0	247.1	108.2	0.0	0.0
0.0	0.0	9	-10.937	-12.377	1.367	17.9	20.0	0.00	3.0	258.3	273.7	116.3	0.0	0.0
0.0	0.0	10	-9.570	-12.416	1.367	17.9	20.0	0.00	0.2	285.8	298.2	123.8	0.0	0.0
0.0	0.0	11	-8.202	-12.389	1.367	17.9	20.0	0.00	-2.6	311.6	320.9	130.8	0.0	0.0
0.0	0.0	12	-6.835	-12.294	1.367	17.9	20.0	0.00	-5.4	329.4	335.8	135.3	0.0	0.0
0.0	0.0	13	-5.467	-12.132	1.367	17.9	20.0	0.00	-8.2	325.1	328.8	132.8	0.0	0.0
0.0	0.0	14	-4.100	-11.900	1.367	17.9	20.0	0.00	-11.0	318.0	319.8	129.8	0.0	0.0
0.0	0.0	15	-2.732	-11.598	1.367	17.9	20.0	0.00	-13.9	309.5	310.3	126.6	0.0	0.0
0.0	0.0	16	-1.365	-11.223	1.367	17.9	20.0	0.00	-16.8	299.1	299.7	123.2	0.0	0.0
0.0	0.0	17	0.003	-10.772	1.367	17.9	20.0	0.00	-19.7	286.7	288.0	119.4	0.0	0.0
0.0	0.0	18	1.370	-10.240	1.367	17.9	20.0	0.00	-22.7	311.4	314.2	128.1	0.0	0.0
0.0	0.0	19	2.738	-9.624	1.367	17.9	20.0	0.00	-25.8	333.2	338.9	136.2	0.0	0.0
0.0	0.0	20	4.105	-8.915	1.367	17.9	20.0	0.00	-29.0	315.7	324.7	132.0	0.0	0.0
0.0	0.0	21	5.473	-8.107	1.367	17.9	20.0	0.00	-32.2	333.1	347.8	139.8	0.0	0.0
0.0	0.0	22	6.840	-7.188	1.367	17.9	20.0	0.00	-35.6	326.0	347.0	140.1	0.0	0.0
0.0	0.0	23	8.208	-6.144	1.367	17.9	20.0	0.00	-39.1	321.1	350.1	141.8	0.0	0.0
0.0	0.0	24	9.575	-4.957	1.367	17.9	20.0	0.00	-42.8	329.1	369.8	149.0	0.0	0.0
0.0	0.0	25	10.943	-3.599	1.367	17.9	20.0	0.00	-46.8	293.5	342.6	141.4	0.0	0.0
0.0	0.0	26	12.310	-2.032	1.367	17.9	20.0	0.00	-51.0	291.2	356.7	147.4	0.0	0.0

0.0	27	13.678	-0.193	1.367	17.9	20.0	0.00	-55.7	276.4	360.5	150.8	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31								
0.0	28	15.045	2.029	1.367	17.9	20.0	0.00	-61.1	223.0	316.7	139.7	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31								
0.0	29	16.413	4.869	1.367	17.9	20.0	0.00	-67.5	186.7	301.2	139.7	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	31								
0.0	30	17.780	9.093	1.367	26.6	12.0	0.00	-76.6	88.0	138.8	94.2	0.0	0.0	0.0
0.0	0.0	0.0	0.0	0.0	1	21								

7395.2

0.0 0.0 0.0 0.0 0.0

Geosynthetic 1
x1 = 0.000 m
y1 = 0.000 m
x2 = 5.000 m
y2 = 0.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 2
x1 = 0.182 m
y1 = 0.500 m
x2 = 5.182 m
y2 = 0.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 3
x1 = 0.364 m
y1 = 1.000 m
x2 = 5.364 m
y2 = 1.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 4
x1 = 2.200 m
y1 = 1.500 m
x2 = 7.200 m
y2 = 1.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 5
x1 = 2.382 m
y1 = 2.000 m
x2 = 7.382 m
y2 = 2.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 6
x1 = 2.564 m
y1 = 2.500 m
x2 = 7.564 m
y2 = 2.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 7
x1 = 4.400 m
y1 = 3.000 m
x2 = 9.400 m
y2 = 3.000 m

$\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 8
 $x_1 = 4.582 \text{ m}$
 $y_1 = 3.500 \text{ m}$
 $x_2 = 9.582 \text{ m}$
 $y_2 = 3.500 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 9
 $x_1 = 4.764 \text{ m}$
 $y_1 = 4.000 \text{ m}$
 $x_2 = 9.764 \text{ m}$
 $y_2 = 4.000 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 10
 $x_1 = 6.600 \text{ m}$
 $y_1 = 4.500 \text{ m}$
 $x_2 = 11.600 \text{ m}$
 $y_2 = 4.500 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 11
 $x_1 = 6.782 \text{ m}$
 $y_1 = 5.000 \text{ m}$
 $x_2 = 11.782 \text{ m}$
 $y_2 = 5.000 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 12
 $x_1 = 6.964 \text{ m}$
 $y_1 = 5.500 \text{ m}$
 $x_2 = 11.964 \text{ m}$
 $y_2 = 5.500 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 13
 $x_1 = 8.800 \text{ m}$
 $y_1 = 6.000 \text{ m}$
 $x_2 = 13.800 \text{ m}$
 $y_2 = 6.000 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 14
 $x_1 = 8.982 \text{ m}$
 $y_1 = 6.500 \text{ m}$
 $x_2 = 13.982 \text{ m}$
 $y_2 = 6.500 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R, d = 58.530 \text{ kN/m}$

Geosynthetic 15
 $x_1 = 9.164 \text{ m}$

y1 = 7.000 m
x2 = 14.164 m
y2 = 7.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 16
x1 = 11.000 m
y1 = 7.500 m
x2 = 16.000 m
y2 = 7.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 17
x1 = 11.182 m
y1 = 8.000 m
x2 = 16.182 m
y2 = 8.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 18
x1 = 11.364 m
y1 = 8.500 m
x2 = 16.364 m
y2 = 8.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 19
x1 = 13.200 m
y1 = 9.000 m
x2 = 18.500 m
y2 = 9.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m
Work. psi(A) = 76.61 °
Limit psi(A) = 75.00 °
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 20
x1 = 13.382 m
y1 = 9.500 m
x2 = 18.682 m
y2 = 9.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m
Work. psi(A) = 76.61 °
Limit psi(A) = 75.00 °
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 21
x1 = 13.564 m
y1 = 10.000 m
x2 = 18.564 m
y2 = 10.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m
Work. psi(A) = 76.61 °

Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 22
 $x1 = 15.400$ m
 $y1 = 10.500$ m
 $x2 = 18.600$ m
 $y2 = 10.500$ m
 $\mu = 0.800$
 $R0 = 58.530$ kN/m
 $R,d = 58.530$ kN/m
Work. $\psi(A) = 76.61^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 23
 $x1 = 15.582$ m
 $y1 = 11.000$ m
 $x2 = 19.082$ m
 $y2 = 11.000$ m
 $\mu = 0.800$
 $R0 = 58.530$ kN/m
 $R,d = 58.530$ kN/m
Work. $\psi(A) = 76.61^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 24
 $x1 = 15.764$ m
 $y1 = 11.500$ m
 $x2 = 19.264$ m
 $y2 = 11.500$ m
 $\mu = 0.800$
 $R0 = 58.530$ kN/m
 $R,d = 58.530$ kN/m
Work. $\psi(A) = 76.61^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 25
 $x1 = 17.600$ m
 $y1 = 12.000$ m
 $x2 = 19.500$ m
 $y2 = 12.000$ m
 $\mu = 0.800$
 $R0 = 58.530$ kN/m
 $R,d = 58.530$ kN/m
Work. $\psi(A) = 76.61^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

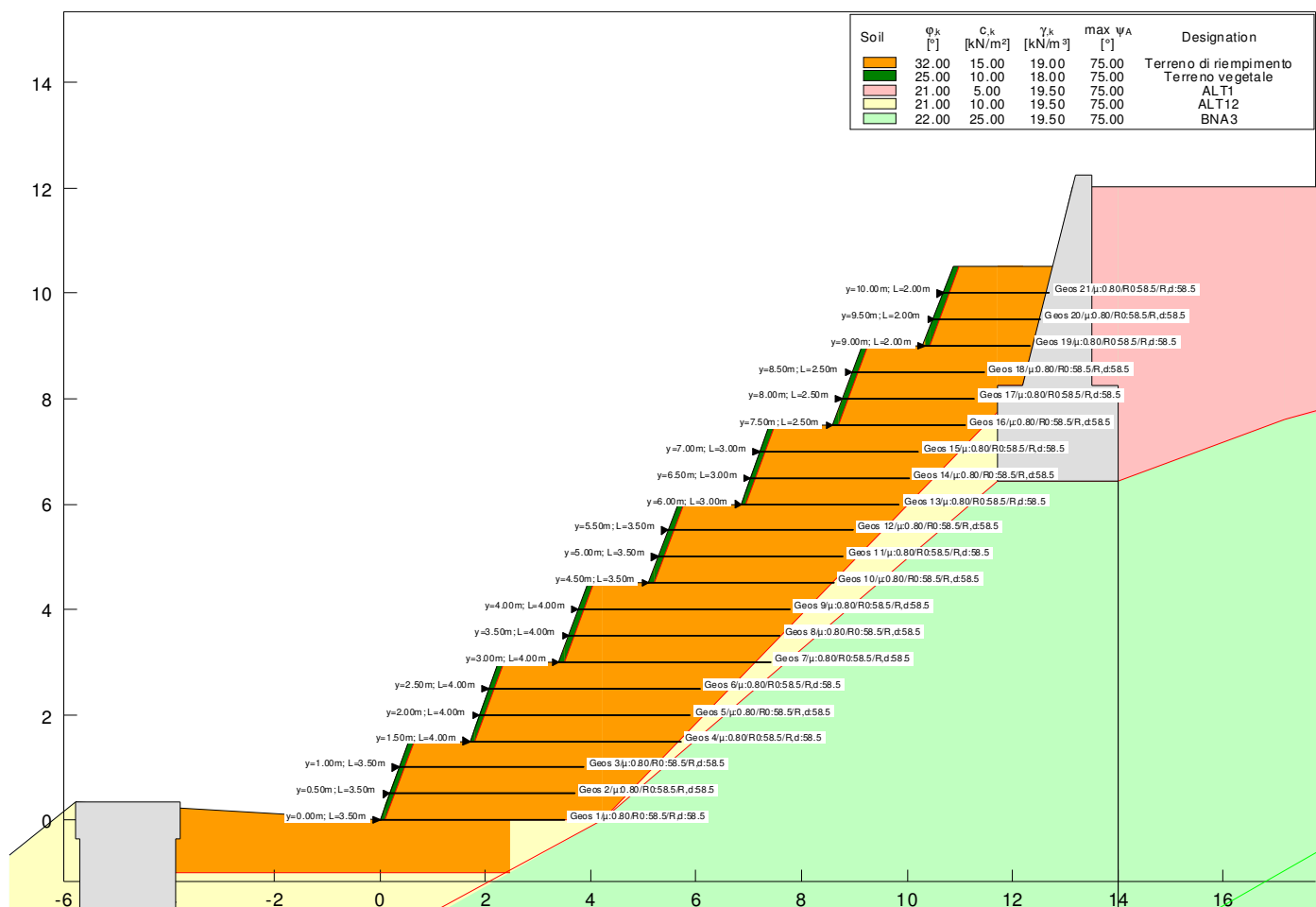
Geosynthetic 26
 $x1 = 17.782$ m
 $y1 = 12.500$ m
 $x2 = 19.682$ m
 $y2 = 12.500$ m
 $\mu = 0.800$
 $R0 = 58.530$ kN/m
 $R,d = 58.530$ kN/m
Work. $\psi(A) = 76.61^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 27
x1 = 17.964 m
y1 = 13.000 m
x2 = 19.864 m
y2 = 13.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m
Work. psi(A) = 76.61 °
Limit psi(A) = 75.00 °
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

SEZIONE4

PROGETTO: ACCADIA
CONFIGURAZIONE STRUTTURALE
SEZIONE 4

Geosintetici di rinforzo				
Strato n°	Tipo	Tamm	Lungh.	Spaz.
[-]	[-]	[kN/m]	[m]	[m]
1-3	Fortrac 110/25-20/30 MPT	58,53	3.50	0.50
4-9	Fortrac 110/25-20/30 MPT	58,53	4.00	0.50
10-12	Fortrac 110/25-20/30 MPT	58,53	3.50	0.50
13-15	Fortrac 110/25-20/30 MPT	58,53	3.00	0.50
16-18	Fortrac 110/25-20/30 MPT	58,53	2.50	0.50
19-21	Fortrac 110/25-20/30 MPT	58,53	2.00	0.50



PROGETTO: ACCADIA
VERIFICA DI STABILITÀ INTERNA E COMPOSTA
SEZIONE 4

NTC2008

Combinazione:

A2+M2+R2

$$E_d \leq (R / \gamma_R)$$

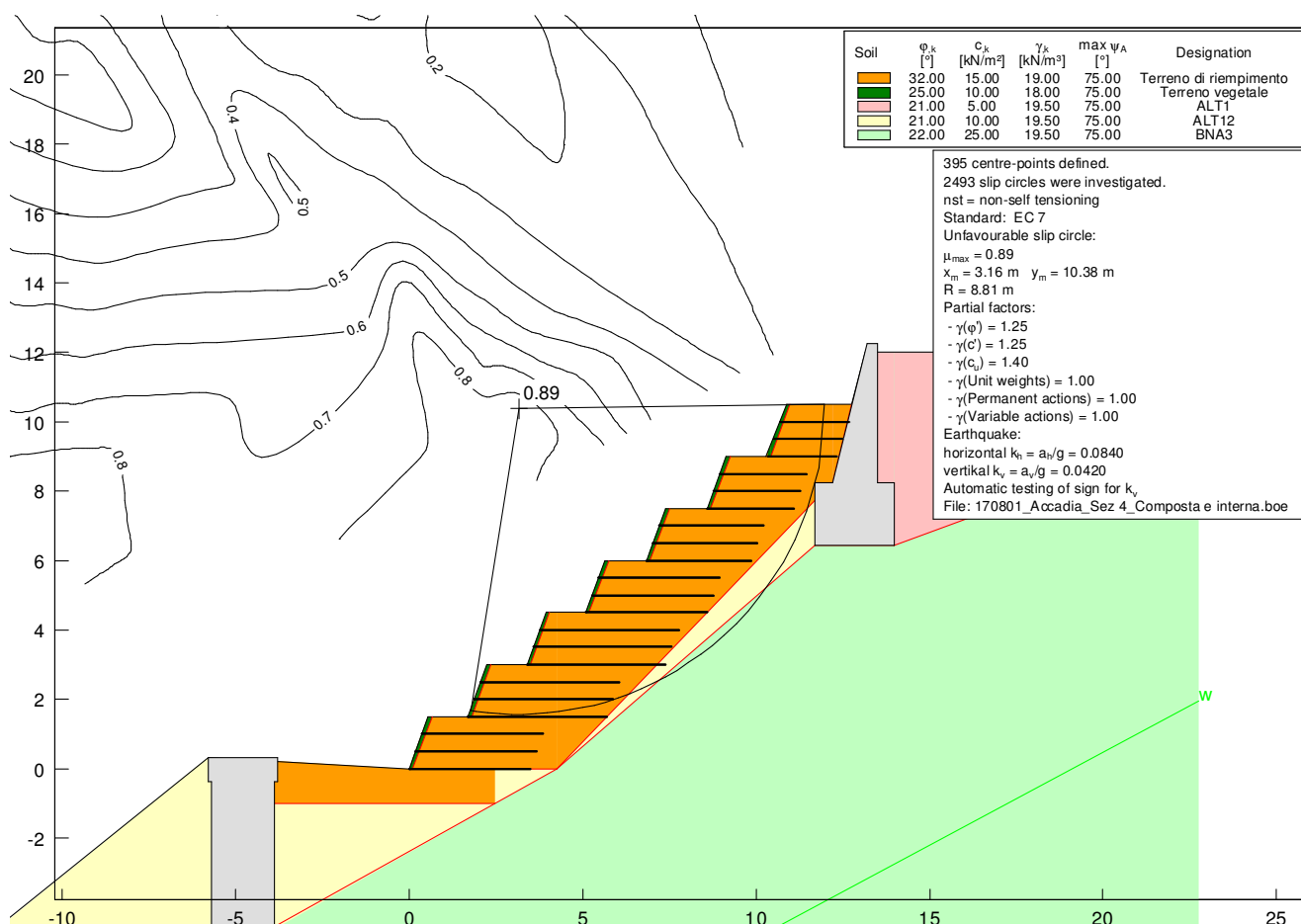
$\gamma_R = 1,1$ (tab. 6.8.I NTC2008)

$\rightarrow R/E_d \geq 1,1 \rightarrow F_s \geq 1,1$.

μ : grado di utilizzo del sistema

$$\mu = 1 / F_s = 0,89$$

$$\rightarrow F_s = 1,12 \geq 1,10$$



Slope stability analysis to EC 7
using circular slip surfaces

Datei: 170801_Accadia_Seiz 4_Composta e interna.boe

Parameter list

phi [°] = friction angle

c [kN/m²] = cohesion

gamma [kN/m³] = unit weight

Max. psi(A) [°] = angle between slip direction of failure mechanism and the tension member

μ [-] = utilisation factor

xm,ym [m] = x and y values of slip circle centre-point

rad [m] = slip circle radius

Partial factors: (GEO-3)

- gam(phi)= 1.25

- gam(c') = 1.25

- gam(cu) = 1.40

- gam(Unit weights) = 1.00

- gam(Permanent actions) = 1.00

- gam(Variable actions) = 1.00

Movement direction of slip body to the Left

Coordinates of surface points

No.	x	y
[-]	[m]	[m]
1	-40.000	-17.345
2	-18.446	-8.074
3	-15.806	-7.816
4	-5.789	0.335
5	-5.789	0.335
6	0.000	0.000
7	0.546	1.500
8	1.700	1.500
9	2.246	3.000
10	3.400	3.000
11	3.946	4.500
12	5.100	4.500
13	5.646	6.000
14	6.846	6.000
15	7.392	7.500
16	8.592	7.500
17	9.138	9.000
18	10.292	9.000
19	10.884	10.500
20	12.764	10.500
21	13.144	12.022
22	22.762	12.022

Characteristic soil properties

Soil	phi,k	c,k	gamma,k	max psi(A)	drained	Designation
[-]	[°]	[kN/m²]	[kN/m³]	[°]		
1	32.00	15.00	19.00	75.00	ja	Terreno di riempimento
2	25.00	10.00	18.00	75.00	ja	Terreno vegetale
3	21.00	5.00	19.50	75.00	ja	ALT1
4	21.00	10.00	19.50	75.00	ja	ALT12
5	22.00	25.00	19.50	75.00	ja	BNA3

Design soil properties

Soil	phi,d	c,d	gamma,d	drained	Designation
[-]	[°]	[kN/m²]	[kN/m³]		
1	26.56	12.00	19.00	ja	Terreno di riempimento
2	20.46	8.00	18.00	ja	Terreno vegetale
3	17.07	4.00	19.50	ja	ALT1
4	17.07	8.00	19.50	ja	ALT12
5	17.91	20.00	19.50	ja	BNA3

Coordinates of layers and soil numbers

No.	x(left)	y(left)	x(right)	y(right)	Soil no.
-----	---------	---------	----------	----------	----------

[-]	[m]	[m]	[m]	[m]	
1	0.000	0.000	0.100	0.000	2
2	0.100	0.000	0.652	1.500	2
3	1.700	1.500	1.800	1.500	2
4	1.800	1.500	2.352	3.000	2
5	3.400	3.000	3.500	3.000	2
6	3.500	3.000	4.052	4.500	2
7	5.100	4.500	5.200	4.500	2
8	5.200	4.500	5.752	6.000	2
9	6.846	6.000	6.946	6.000	2
10	6.946	6.000	7.498	7.500	2
11	8.592	7.500	8.692	7.500	2
12	8.692	7.500	9.244	9.000	2
13	10.338	9.000	10.438	9.000	2
14	10.438	9.000	10.990	10.500	2
15	0.100	0.000	4.239	0.000	1
16	-3.889	-1.000	2.476	-1.000	1
17	4.239	0.000	11.700	7.743	1
18	11.700	8.243	12.200	8.243	1
19	12.200	8.243	12.764	10.500	1
20	-40.000	-22.718	-15.254	-11.057	4
21	-15.254	-11.057	4.239	0.000	4
22	4.239	0.000	11.700	6.443	4
23	11.700	6.443	14.000	6.443	3
24	14.000	6.443	17.172	7.612	3
25	17.172	7.612	22.762	8.832	3
26	-40.000	-40.000	22.762	-40.000	5

Coordinates of pwp polygon course

No.	x	y
[-]	[m]	[m]
1	-40.000	-27.268
2	-11.593	-16.727
3	22.762	1.961

Geosynthetics

Adhesive force f calculated with:

$$f = \mu \cdot \tan(\phi) \cdot \sigma'$$

 μ [-] = reduction factor for friction between ground and geosynthetics

 σ' [kN/m²] = effekive Spannung

R0 [kN/m] = design force at connection

L0 [m] = fold-back length

R,d [kN/m] = acceptable design force

Nr.	x1	y1	x2	y2	μ	R0	R, d
[-]	[m]	[m]	[m]	[m]	[-]	[kN/m]	[kN/m]
1	0.00	0.00	3.50	0.00	0.800	58.53	58.53
2	0.18	0.50	3.68	0.50	0.800	58.53	58.53
3	0.36	1.00	3.86	1.00	0.800	58.53	58.53
4	1.70	1.50	5.70	1.50	0.800	58.53	58.53
5	1.88	2.00	5.88	2.00	0.800	58.53	58.53
6	2.06	2.50	6.06	2.50	0.800	58.53	58.53
7	3.40	3.00	7.40	3.00	0.800	58.53	58.53
8	3.58	3.50	7.58	3.50	0.800	58.53	58.53
9	3.76	4.00	7.76	4.00	0.800	58.53	58.53
10	5.10	4.50	8.60	4.50	0.800	58.53	58.53
11	5.28	5.00	8.78	5.00	0.800	58.53	58.53
12	5.46	5.50	8.96	5.50	0.800	58.53	58.53
13	6.85	6.00	9.85	6.00	0.800	58.53	58.53
14	7.03	6.50	10.03	6.50	0.800	58.53	58.53
15	7.21	7.00	10.21	7.00	0.800	58.53	58.53
16	8.59	7.50	11.09	7.50	0.800	58.53	58.53
17	8.77	8.00	11.27	8.00	0.800	58.53	58.53
18	8.96	8.50	11.46	8.50	0.800	58.53	58.53
19	10.34	9.00	12.34	9.00	0.800	58.53	58.53
20	10.52	9.50	12.52	9.50	0.800	58.53	58.53
21	10.70	10.00	12.70	10.00	0.800	58.53	58.53

Structural element 1

No.	x	y
[-]	[m]	[m]
1	-5.79	0.34
2	-3.79	0.34
3	-3.79	-0.37
4	-3.89	-0.37
5	-3.89	-11.67
6	-5.69	-11.67
7	-5.69	-0.37
8	-5.79	-0.37

Structural element 2

No.	x	y
[-]	[m]	[m]
1	13.20	12.24
2	13.50	12.24
3	13.50	8.24
4	14.00	8.24
5	14.00	6.44
6	11.70	6.44
7	11.70	8.24
8	12.20	8.24

Earthquake

horizontal $k_h = a_h/g = 0.0840$

vertical $k_v = a_v/g = 0.0420$

Automatic testing of sign for k_v
 k_v (governing) = -0.0420

(a_h = horizontal seismic acceleration in m/s^2)

(a_v = vertical seismic acceleration in m/s^2)

(g = gravitational acceleration = $9.81 m/s^2$)

Water level in front of slope left [m] = -20.00

Water level in front of slope right [m] = -20.00

Gamma water [kN/m^3] = 10.000

Calculation with consideration of passive earth pressure wedge

Slip circle no. 202

 $\mu = 0.8865 = [M(G_i) + M(S)] / [M(T_i) + M(R)]$
 $x_m = 3.1645$
 $y_m = 10.3777$

Radius = 8.8114

Numerator = 3279.8794

Denominator = 3699.9687

 $M(T_i) = 3699.9687$
 $M(R) = 0.0000$
 $M(G_i) = 3002.4360$
 $M(S) = 277.4434$

Slice values

 $x = x$ (slice toe)

 $y = y$ (slice toe)

 b = slice width

 ϕ = friction angle

 c = cohesion

PWP = pore water press. coeff.

 α = inclination of slice

 g = weight

 n = normal force

 t = tangential force

 $F_{Ai}/F_{A0i}/R_{si}$ = see Equations (7) and (8) in DIN 4084:2009

 p_w = pore water pressure

 $p_w(con)$ = excess pwp due to consolidation

 w_v = vertical water pressure

 p_{st} = permanent loads and footing

 p_v = live loads

 f_{akpv} = factor for live loads

So No. = soil number															
	Nr.	x	y	b	phi,d	c,d	PWD	tet	g,k	n	t	FAi	FA0i	Rsi	
pw	pw(kon)	wv	pst,d	pv,d	fakpv	Bo-Nr.									
[kN/m]	[kN/m]	[m]	[m]	[m]	[°]	[kN/m²]	[m]	[°]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	
0.0	1	1.935	1.653	0.340	26.6	12.0	0.00	8.0	3.1	3.5	6.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	2	2.275	1.611	0.340	26.6	12.0	0.00	5.8	8.9	9.8	9.2	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	3	2.616	1.583	0.340	26.6	12.0	0.00	3.6	9.2	9.8	9.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	4	2.956	1.569	0.340	26.6	12.0	0.00	1.4	9.3	9.7	9.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	5	3.297	1.567	0.340	26.6	12.0	0.00	-0.9	9.3	9.6	8.9	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	6	3.637	1.579	0.340	26.6	12.0	0.00	-3.1	13.3	13.6	10.8	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	7	3.977	1.604	0.340	26.6	12.0	0.00	-5.3	18.7	18.8	13.3	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	15									
0.0	8	4.318	1.642	0.340	26.6	12.0	0.00	-7.5	18.5	18.4	13.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	17									
0.0	9	4.658	1.694	0.340	26.6	12.0	0.00	-9.8	18.1	17.8	12.8	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	17									
0.0	10	4.998	1.759	0.340	26.6	12.0	0.00	-12.0	17.7	17.3	12.4	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	17									
0.0	11	5.339	1.839	0.340	26.6	12.0	0.00	-14.3	21.4	20.6	14.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	17									
0.0	12	5.679	1.933	0.340	26.6	12.0	0.00	-16.6	26.2	25.2	16.4	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	17									
0.0	13	6.019	2.042	0.340	26.6	12.0	0.00	-18.9	25.6	24.5	16.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	0	17									
0.0	14	6.360	2.166	0.340	17.1	8.0	0.00	-21.3	24.8	25.1	10.3	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	15	6.700	2.307	0.340	17.1	8.0	0.00	-23.7	23.9	24.3	10.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	16	7.041	2.465	0.340	17.1	8.0	0.00	-26.1	26.3	26.9	10.9	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	17	7.381	2.641	0.340	17.9	20.0	0.00	-28.6	31.2	32.1	17.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	18	7.721	2.836	0.340	17.9	20.0	0.00	-31.1	30.3	31.4	16.9	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	19	8.062	3.053	0.340	17.9	20.0	0.00	-33.8	28.9	30.4	16.7	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	20	8.402	3.292	0.340	17.9	20.0	0.00	-36.5	27.4	29.3	16.5	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	21	8.742	3.557	0.340	17.9	20.0	0.00	-39.3	28.3	30.8	17.1	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	22	9.083	3.850	0.340	17.9	20.0	0.00	-42.2	32.4	36.2	19.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	23	9.423	4.175	0.340	17.9	20.0	0.00	-45.3	31.4	36.1	19.2	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	24	9.763	4.539	0.340	17.9	20.0	0.00	-48.5	29.1	34.5	18.9	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	25	10.104	4.948	0.340	17.9	20.0	0.00	-52.0	26.4	32.7	18.6	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	26									
0.0	26	10.444	5.413	0.340	17.1	8.0	0.00	-55.7	25.7	34.0	13.9	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	27	10.785	5.953	0.340	17.1	8.0	0.00	-59.9	27.8	39.3	15.8	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	28	11.125	6.600	0.340	17.1	8.0	0.00	-64.6	25.3	39.1	16.0	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	29	11.465	7.422	0.340	17.1	8.0	0.00	-70.4	19.9	35.1	15.4	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	22									
0.0	30	11.806	8.654	0.340	26.6	12.0	0.00	-78.7	11.9	19.7	16.3	0.0	0.0	0.0	
0.0	0.0	0.0	0.0	0.0	1	18									

650.3

Geosynthetic 1
x1 = 0.000 m
y1 = 0.000 m
x2 = 3.500 m
y2 = 0.000 m
μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 2
x1 = 0.182 m
y1 = 0.500 m

$x2 = 3.682 \text{ m}$
 $y2 = 0.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 3
 $x1 = 0.364 \text{ m}$
 $y1 = 1.000 \text{ m}$
 $x2 = 3.864 \text{ m}$
 $y2 = 1.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 4
 $x1 = 1.700 \text{ m}$
 $y1 = 1.500 \text{ m}$
 $x2 = 5.700 \text{ m}$
 $y2 = 1.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 5
 $x1 = 1.882 \text{ m}$
 $y1 = 2.000 \text{ m}$
 $x2 = 5.882 \text{ m}$
 $y2 = 2.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 6
 $x1 = 2.064 \text{ m}$
 $y1 = 2.500 \text{ m}$
 $x2 = 6.064 \text{ m}$
 $y2 = 2.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 7
 $x1 = 3.400 \text{ m}$
 $y1 = 3.000 \text{ m}$
 $x2 = 7.400 \text{ m}$
 $y2 = 3.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 8
 $x1 = 3.582 \text{ m}$
 $y1 = 3.500 \text{ m}$
 $x2 = 7.582 \text{ m}$
 $y2 = 3.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 9
 $x1 = 3.764 \text{ m}$
 $y1 = 4.000 \text{ m}$
 $x2 = 7.764 \text{ m}$
 $y2 = 4.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$

Geosynthetic 10
x1 = 5.100 m
y1 = 4.500 m
x2 = 8.600 m
y2 = 4.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 11
x1 = 5.282 m
y1 = 5.000 m
x2 = 8.782 m
y2 = 5.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 12
x1 = 5.464 m
y1 = 5.500 m
x2 = 8.964 m
y2 = 5.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 13
x1 = 6.850 m
y1 = 6.000 m
x2 = 9.850 m
y2 = 6.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 14
x1 = 7.031 m
y1 = 6.500 m
x2 = 10.031 m
y2 = 6.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 15
x1 = 7.211 m
y1 = 7.000 m
x2 = 10.211 m
y2 = 7.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 16
x1 = 8.592 m
y1 = 7.500 m
x2 = 11.092 m
y2 = 7.500 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 17
x1 = 8.774 m
y1 = 8.000 m
x2 = 11.274 m
y2 = 8.000 m
 $\mu = 0.800$

$R_0 = 58.530 \text{ kN/m}$
 $R_{,d} = 58.530 \text{ kN/m}$

Geosynthetic 18
 $x_1 = 8.956 \text{ m}$
 $y_1 = 8.500 \text{ m}$
 $x_2 = 11.456 \text{ m}$
 $y_2 = 8.500 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R_{,d} = 58.530 \text{ kN/m}$

Geosynthetic 19
 $x_1 = 10.338 \text{ m}$
 $y_1 = 9.000 \text{ m}$
 $x_2 = 12.338 \text{ m}$
 $y_2 = 9.000 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R_{,d} = 58.530 \text{ kN/m}$
Work. $\psi(A) = 78.72^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 20
 $x_1 = 10.520 \text{ m}$
 $y_1 = 9.500 \text{ m}$
 $x_2 = 12.520 \text{ m}$
 $y_2 = 9.500 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R_{,d} = 58.530 \text{ kN/m}$
Work. $\psi(A) = 78.72^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

Geosynthetic 21
 $x_1 = 10.702 \text{ m}$
 $y_1 = 10.000 \text{ m}$
 $x_2 = 12.702 \text{ m}$
 $y_2 = 10.000 \text{ m}$
 $\mu = 0.800$
 $R_0 = 58.530 \text{ kN/m}$
 $R_{,d} = 58.530 \text{ kN/m}$
Work. $\psi(A) = 78.72^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is not self-tensioning nach DIN 4084:2009
Geosynthetic therefore makes no impact (DIN 4084:2009 7.2.3.4)

PROGETTO: ACCADIA
VERIFICA DI STABILITÀ SEZIONE 4
Verifica di stabilità globale

NTC2008

Combinazione:

A2+M2+R2

$$E_d \leq (R / \gamma_R)$$

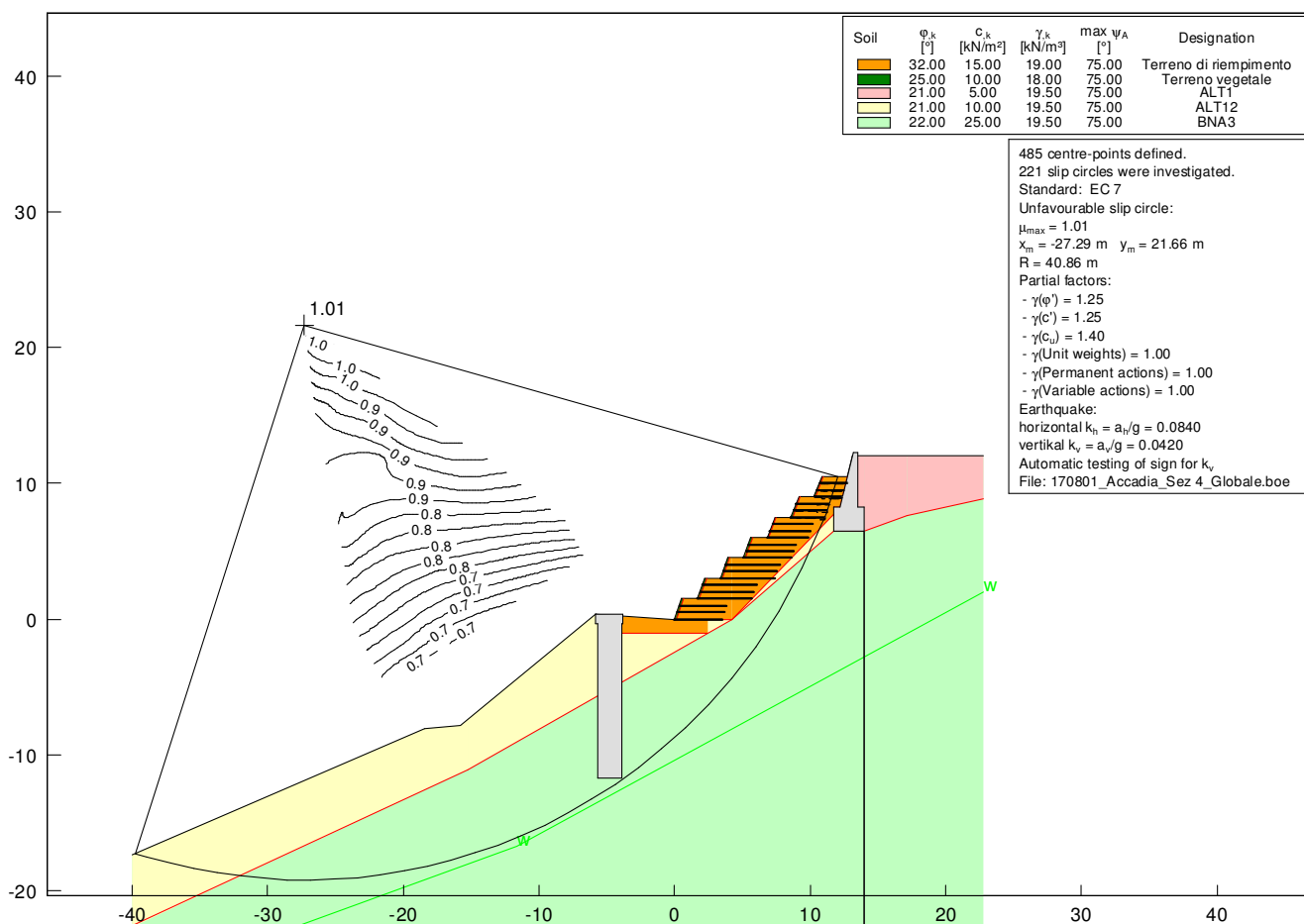
$\gamma_R = 1,1$ (tab. 6.8.I NTC2008)

$\rightarrow R/E_d \geq 1,1 \rightarrow F_s \geq 1,1$.

μ : grado di utilizzo del sistema

$$\mu = 1 / F_s = 1,01$$

$$\rightarrow F_s = 0,99 < 1,10$$



Slope stability analysis to EC 7

using circular slip surfaces

Datei: 170801_Accadia-Sez 4_Globale.boe

Parameter list

phi [°] = friction angle

c [kN/m²] = cohesion

gamma [kN/m³] = unit weight

Max. psi(A) [°] = angle between slip direction of failure mechanism and the tension member

μ [-] = utilisation factor

xm,ym [m] = x and y values of slip circle centre-point

rad [m] = slip circle radius

Partial factors: (GEO-3)

- gam(phi) = 1.25

- gam(c') = 1.25

- gam(cu) = 1.40

- gam(Unit weights) = 1.00

- gam(Permanent actions) = 1.00

- gam(Variable actions) = 1.00

Movement direction of slip body to the Left

Coordinates of surface points

No. [-]	x [m]	y [m]
1	-40.000	-17.345
2	-18.446	-8.074
3	-15.806	-7.816
4	-5.789	0.335
5	-5.789	0.335
6	0.000	0.000
7	0.546	1.500
8	1.700	1.500
9	2.246	3.000
10	3.400	3.000
11	3.946	4.500
12	5.100	4.500
13	5.646	6.000
14	6.846	6.000
15	7.392	7.500
16	8.592	7.500
17	9.138	9.000
18	10.292	9.000
19	10.884	10.500
20	12.764	10.500
21	13.144	12.022
22	22.762	12.022

Characteristic soil properties

Soil [-]	phi,k [°]	c,k [kN/m²]	gamma,k [kN/m³]	max psi(A) [°]	drained	Designation
1	32.00	15.00	19.00	75.00	ja	Terreno di riempimento
2	25.00	10.00	18.00	75.00	ja	Terreno vegetale
3	21.00	5.00	19.50	75.00	ja	ALT1
4	21.00	10.00	19.50	75.00	ja	ALT12
5	22.00	25.00	19.50	75.00	ja	BNA3

Design soil properties

Soil [-]	phi,d [°]	c,d [kN/m²]	gamma,d [kN/m³]	drained	Designation
1	26.56	12.00	19.00	ja	Terreno di riempimento
2	20.46	8.00	18.00	ja	Terreno vegetale
3	17.07	4.00	19.50	ja	ALT1
4	17.07	8.00	19.50	ja	ALT12
5	17.91	20.00	19.50	ja	BNA3

Coordinates of layers and soil numbers

No. [-]	x(left) [m]	y(left) [m]	x(right) [m]	y(right) [m]	Soil no.
------------	----------------	----------------	-----------------	-----------------	----------

1	0.000	0.000	0.100	0.000	2
2	0.100	0.000	0.652	1.500	2
3	1.700	1.500	1.800	1.500	2
4	1.800	1.500	2.352	3.000	2
5	3.400	3.000	3.500	3.000	2
6	3.500	3.000	4.052	4.500	2
7	5.100	4.500	5.200	4.500	2
8	5.200	4.500	5.752	6.000	2
9	6.846	6.000	6.946	6.000	2
10	6.946	6.000	7.498	7.500	2
11	8.592	7.500	8.692	7.500	2
12	8.692	7.500	9.244	9.000	2
13	10.338	9.000	10.438	9.000	2
14	10.438	9.000	10.990	10.500	2
15	0.100	0.000	4.239	0.000	1
16	-3.889	-1.000	2.476	-1.000	1
17	4.239	0.000	11.700	7.743	1
18	11.700	8.243	12.200	8.243	1
19	12.200	8.243	12.764	10.500	1
20	-40.000	-22.718	-15.254	-11.057	4
21	-15.254	-11.057	4.239	0.000	4
22	4.239	0.000	11.700	6.443	4
23	11.700	6.443	14.000	6.443	3
24	14.000	6.443	17.172	7.612	3
25	17.172	7.612	22.762	8.832	3
26	-40.000	-40.000	22.762	-40.000	5

Coordinates of pwp polygon course

No.	x	y
[-]	[m]	[m]
1	-40.000	-27.268
2	-11.593	-16.727
3	22.762	1.961

Geosynthetics

Adhesive force f calculated with:

$$f = \mu \cdot \tan(\phi) \cdot \sigma'$$

 μ [-] = reduction factor for friction between ground and geosynthetics

 σ' [kN/m²] = effekive Spannung

R0 [kN/m] = design force at connection

L0 [m] = fold-back length

R,d [kN/m] = acceptable design force

Nr.	x1	y1	x2	y2	μ	R0	R, d
[-]	[m]	[m]	[m]	[m]	[-]	[kN/m]	[kN/m]
1	0.00	0.00	3.50	0.00	0.800	58.53	58.53
2	0.18	0.50	3.68	0.50	0.800	58.53	58.53
3	0.36	1.00	3.86	1.00	0.800	58.53	58.53
4	1.70	1.50	5.70	1.50	0.800	58.53	58.53
5	1.88	2.00	5.88	2.00	0.800	58.53	58.53
6	2.06	2.50	6.06	2.50	0.800	58.53	58.53
7	3.40	3.00	7.40	3.00	0.800	58.53	58.53
8	3.58	3.50	7.58	3.50	0.800	58.53	58.53
9	3.76	4.00	7.76	4.00	0.800	58.53	58.53
10	5.10	4.50	8.60	4.50	0.800	58.53	58.53
11	5.28	5.00	8.78	5.00	0.800	58.53	58.53
12	5.46	5.50	8.96	5.50	0.800	58.53	58.53
13	6.85	6.00	9.85	6.00	0.800	58.53	58.53
14	7.03	6.50	10.03	6.50	0.800	58.53	58.53
15	7.21	7.00	10.21	7.00	0.800	58.53	58.53
16	8.59	7.50	11.09	7.50	0.800	58.53	58.53
17	8.77	8.00	11.27	8.00	0.800	58.53	58.53
18	8.96	8.50	11.46	8.50	0.800	58.53	58.53
19	10.34	9.00	12.34	9.00	0.800	58.53	58.53
20	10.52	9.50	12.52	9.50	0.800	58.53	58.53
21	10.70	10.00	12.70	10.00	0.800	58.53	58.53

Structural element 1

No.	x	y
-----	---	---

[-]	[m]	[m]
1	-5.79	0.34
2	-3.79	0.34
3	-3.79	-0.37
4	-3.89	-0.37
5	-3.89	-11.67
6	-5.69	-11.67
7	-5.69	-0.37
8	-5.79	-0.37

Structural element 2

No.	x	y
[-]	[m]	[m]
1	13.20	12.24
2	13.50	12.24
3	13.50	8.24
4	14.00	8.24
5	14.00	6.44
6	11.70	6.44
7	11.70	8.24
8	12.20	8.24

Structural element 3

No.	x	y
[-]	[m]	[m]
1	14.00	6.44
2	14.00	-40.00

Earthquake

horizontal $k_h = a_h/g = 0.0840$

vertical $k_v = a_v/g = 0.0420$

Automatic testing of sign for k_v
 k_v (governing) = -0.0420

(a_h = horizontal seismic acceleration in m/s^2)

(a_v = vertical seismic acceleration in m/s^2)

(g = gravitational acceleration = $9.81 m/s^2$)

Water level in front of slope left [m] = -20.00

Water level in front of slope right [m] = -20.00

Gamma water $[kN/m^3] = 10.000$

Calculation with consideration of passive earth pressure wedge

Slip circle no. 334

 $\mu = 1.0134 = [M(G_i) + M(S)] / [M(T_i) + M(R)]$
 $x_m = -27.2939$
 $y_m = 21.6592$

Radius = 40.8602

Numerator = 158007.6159

Denominator = 155925.9346

 $M(T_i) = 155925.9346$
 $M(R) = 0.0000$
 $M(G_i) = 136932.9903$
 $M(S) = 21074.6256$

Slice values

 $x = x$ (slice toe)

 $y = y$ (slice toe)

 b = slice width

 ϕ = friction angle

 c = cohesion

PWP = pore water press. coeff.

 θ = inclination of slice

 g = weight

 n = normal force

 t = tangential force

 $F_{Ai}/F_{A0i}/R_{si}$ = see Equations (7) and (8) in DIN 4084:2009

 p_w = pore water pressure

pw(kon) = excess pwp due to consolidation
 wv = vertical water pressure
 pst = permanent loads and footing
 pv = live loads
 fakpv = factor for live loads
 So No. = soil number

Nr.	x	y	b	phi,d	c,d	PWD	tet	g,k	n	t	FAi	FA0i	Rsi
pw	pw(kon)	wv	pst,d	pv,d	fakpv	Bo-Nr.							
[kN/m]	[kN/m]	[m]	[kN/m]	[kN/m]	[°]	[kN/m²]	[-]	[°]	[kN/m]	[kN/m]	[kN/m]	[kN/m]	[kN/m]
0.0	0.0	1 -38.912	-17.514	1.726	17.1	8.0	0.00	16.5	21.5	25.7	23.8	0.0	0.0
0.0	0.0	2 -37.186	-17.986	1.726	17.1	8.0	0.00	14.0	62.3	72.6	37.7	0.0	0.0
0.0	0.0	3 -35.460	-18.377	1.726	17.1	8.0	0.00	11.5	100.5	114.1	50.1	0.0	0.0
0.0	0.0	4 -33.733	-18.690	1.726	17.1	8.0	0.00	9.1	136.0	151.0	61.1	0.0	0.0
0.0	0.0	5 -32.007	-18.928	1.726	17.1	8.0	0.00	6.6	169.0	184.0	70.9	0.0	0.0
0.0	0.0	6 -30.281	-19.092	1.726	17.9	20.0	0.00	4.2	199.5	213.6	104.5	0.0	0.0
0.0	0.0	7 -28.555	-19.182	1.726	17.9	20.0	0.00	1.8	227.6	239.6	112.4	0.0	0.0
0.0	0.0	8 -26.828	-19.198	1.726	17.9	20.0	0.00	-0.7	253.1	262.8	119.3	0.0	0.0
0.0	0.0	9 -25.102	-19.142	1.726	17.9	20.0	0.00	-3.1	276.2	283.2	125.5	0.0	0.0
0.0	0.0	10 -23.376	-19.013	1.726	17.9	20.0	0.00	-5.5	296.9	301.2	131.0	0.0	0.0
0.0	0.0	11 -21.649	-18.809	1.726	17.9	20.0	0.00	-7.9	315.0	316.9	135.8	0.0	0.0
0.0	0.0	12 -19.923	-18.531	1.726	17.9	20.0	0.00	-10.4	330.6	330.4	139.9	0.0	0.0
0.0	0.0	13 -18.197	-18.175	1.726	17.9	20.0	0.00	-12.9	340.9	339.0	142.5	0.0	0.0
0.0	0.0	14 -16.471	-17.741	1.726	17.9	20.0	0.00	-15.4	331.9	329.1	139.2	0.0	0.0
0.0	0.0	15 -14.744	-17.226	1.726	17.9	20.0	0.00	-17.9	345.9	342.5	143.5	0.0	0.0
0.0	0.0	16 -13.018	-16.626	1.726	17.9	20.0	0.00	-20.4	372.9	369.6	152.3	0.0	0.0
0.0	0.0	17 -11.292	-15.937	1.726	17.9	20.0	0.00	-23.1	397.0	394.6	160.5	0.0	0.0
0.0	0.0	18 -9.566	-15.155	1.726	17.9	20.0	0.00	-25.7	418.0	417.5	168.1	0.0	0.0
0.0	0.0	19 -7.839	-14.272	1.726	17.9	20.0	0.00	-28.4	435.6	438.4	175.0	0.0	0.0
0.0	0.0	20 -6.113	-13.283	1.726	17.9	20.0	0.00	-31.2	449.5	457.0	181.4	0.0	0.0
0.0	0.0	21 -4.387	-12.176	1.726	17.9	20.0	0.00	-34.1	418.4	430.9	173.4	0.0	0.0
0.0	0.0	22 -2.660	-10.941	1.726	17.9	20.0	0.00	-37.1	372.5	389.9	160.7	0.0	0.0
0.0	0.0	23 -0.934	-9.561	1.726	17.9	20.0	0.00	-40.2	322.8	344.8	146.8	0.0	0.0
0.0	0.0	24 0.792	-8.018	1.726	17.9	20.0	0.00	-43.4	318.2	348.5	148.9	0.0	0.0
0.0	0.0	25 2.518	-6.283	1.726	17.9	20.0	0.00	-46.9	309.9	349.9	150.5	0.0	0.0
0.0	0.0	26 4.245	-4.319	1.726	17.9	20.0	0.00	-50.5	293.0	343.6	149.9	0.0	0.0
0.0	0.0	27 5.971	-2.069	1.726	17.9	20.0	0.00	-54.5	268.0	329.5	147.3	0.0	0.0
0.0	0.0	28 7.697	0.560	1.726	17.9	20.0	0.00	-58.9	230.2	301.1	140.6	0.0	0.0
0.0	0.0	29 9.423	3.732	1.726	17.9	20.0	0.00	-64.0	174.2	247.6	127.1	0.0	0.0
0.0	0.0	30 11.150	7.816	1.726	26.6	12.0	0.00	-70.2	88.0	112.5	81.6	43.0	0.0
0.0	0.0	0.0	0.0	0.0	1	17							

0.0	0.0	0.0	0.0	0.0	8275.2								

Geosynthetic 1
 x1 = 0.000 m
 y1 = 0.000 m
 x2 = 3.500 m
 y2 = 0.000 m
 μ = 0.800
 R0 = 58.530 kN/m

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 2

$x1 = 0.182 \text{ m}$

$y1 = 0.500 \text{ m}$

$x2 = 3.682 \text{ m}$

$y2 = 0.500 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 3

$x1 = 0.364 \text{ m}$

$y1 = 1.000 \text{ m}$

$x2 = 3.864 \text{ m}$

$y2 = 1.000 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 4

$x1 = 1.700 \text{ m}$

$y1 = 1.500 \text{ m}$

$x2 = 5.700 \text{ m}$

$y2 = 1.500 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 5

$x1 = 1.882 \text{ m}$

$y1 = 2.000 \text{ m}$

$x2 = 5.882 \text{ m}$

$y2 = 2.000 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 6

$x1 = 2.064 \text{ m}$

$y1 = 2.500 \text{ m}$

$x2 = 6.064 \text{ m}$

$y2 = 2.500 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 7

$x1 = 3.400 \text{ m}$

$y1 = 3.000 \text{ m}$

$x2 = 7.400 \text{ m}$

$y2 = 3.000 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 8

$x1 = 3.582 \text{ m}$

$y1 = 3.500 \text{ m}$

$x2 = 7.582 \text{ m}$

$y2 = 3.500 \text{ m}$

$\mu = 0.800$

$R0 = 58.530 \text{ kN/m}$

$R, d = 58.530 \text{ kN/m}$

Geosynthetic 9

$x1 = 3.764 \text{ m}$

$y1 = 4.000 \text{ m}$

$x2 = 7.764 \text{ m}$

y2 = 4.000 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 10
x1 = 5.100 m
y1 = 4.500 m
x2 = 8.600 m
y2 = 4.500 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 11
x1 = 5.282 m
y1 = 5.000 m
x2 = 8.782 m
y2 = 5.000 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 12
x1 = 5.464 m
y1 = 5.500 m
x2 = 8.964 m
y2 = 5.500 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 13
x1 = 6.850 m
y1 = 6.000 m
x2 = 9.850 m
y2 = 6.000 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 14
x1 = 7.031 m
y1 = 6.500 m
x2 = 10.031 m
y2 = 6.500 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 15
x1 = 7.211 m
y1 = 7.000 m
x2 = 10.211 m
y2 = 7.000 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m

Geosynthetic 16
x1 = 8.592 m
y1 = 7.500 m
x2 = 11.092 m
y2 = 7.500 m
 μ = 0.800
R0 = 58.530 kN/m
R,d = 58.530 kN/m
Work. psi(A) = 70.20 °
Limit psi(A) = 75.00 °

Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 2.615 kN/m
Bonded length = 0.06 m
 $F(AL) = 117.85 \text{ kN/m}$
 $F(AR) = 2.61 \text{ kN/m}$

Geosynthetic 17
 $x1 = 8.774 \text{ m}$
 $y1 = 8.000 \text{ m}$
 $x2 = 11.274 \text{ m}$
 $y2 = 8.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 70.20^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 2.217 kN/m
Bonded length = 0.06 m
 $F(AL) = 106.89 \text{ kN/m}$
 $F(AR) = 2.22 \text{ kN/m}$

Geosynthetic 18
 $x1 = 8.956 \text{ m}$
 $y1 = 8.500 \text{ m}$
 $x2 = 11.456 \text{ m}$
 $y2 = 8.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 70.20^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 2.025 kN/m
Bonded length = 0.07 m
 $F(AL) = 94.32 \text{ kN/m}$
 $F(AR) = 2.03 \text{ kN/m}$

Geosynthetic 19
 $x1 = 10.338 \text{ m}$
 $y1 = 9.000 \text{ m}$
 $x2 = 12.338 \text{ m}$
 $y2 = 9.000 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 70.20^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 17.829 kN/m
Bonded length = 0.78 m
 $F(AL) = 80.36 \text{ kN/m}$
 $F(AR) = 17.83 \text{ kN/m}$

Geosynthetic 20
 $x1 = 10.520 \text{ m}$
 $y1 = 9.500 \text{ m}$
 $x2 = 12.520 \text{ m}$
 $y2 = 9.500 \text{ m}$
 $\mu = 0.800$
 $R0 = 58.530 \text{ kN/m}$
 $R,d = 58.530 \text{ kN/m}$
Work. $\psi(A) = 70.20^\circ$
Limit $\psi(A) = 75.00^\circ$
Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 12.112 kN/m
Bonded length = 0.80 m
 $F(AL) = 73.97 \text{ kN/m}$
 $F(AR) = 12.11 \text{ kN/m}$

Geosynthetic 21
x1 = 10.702 m
y1 = 10.000 m
x2 = 12.702 m
y2 = 10.000 m
 $\mu = 0.800$
R0 = 58.530 kN/m
R,d = 58.530 kN/m
Work. psi(A) = 70.20 °
Limit psi(A) = 75.00 °
Geosynthetic is self-tensioning (DIN 4084:2009 7.2.3.4).
Activated force = 6.166 kN/m
Bonded length = 0.83 m
F(AL) = 66.64 kN/m
F(AR) = 6.17 kN/m